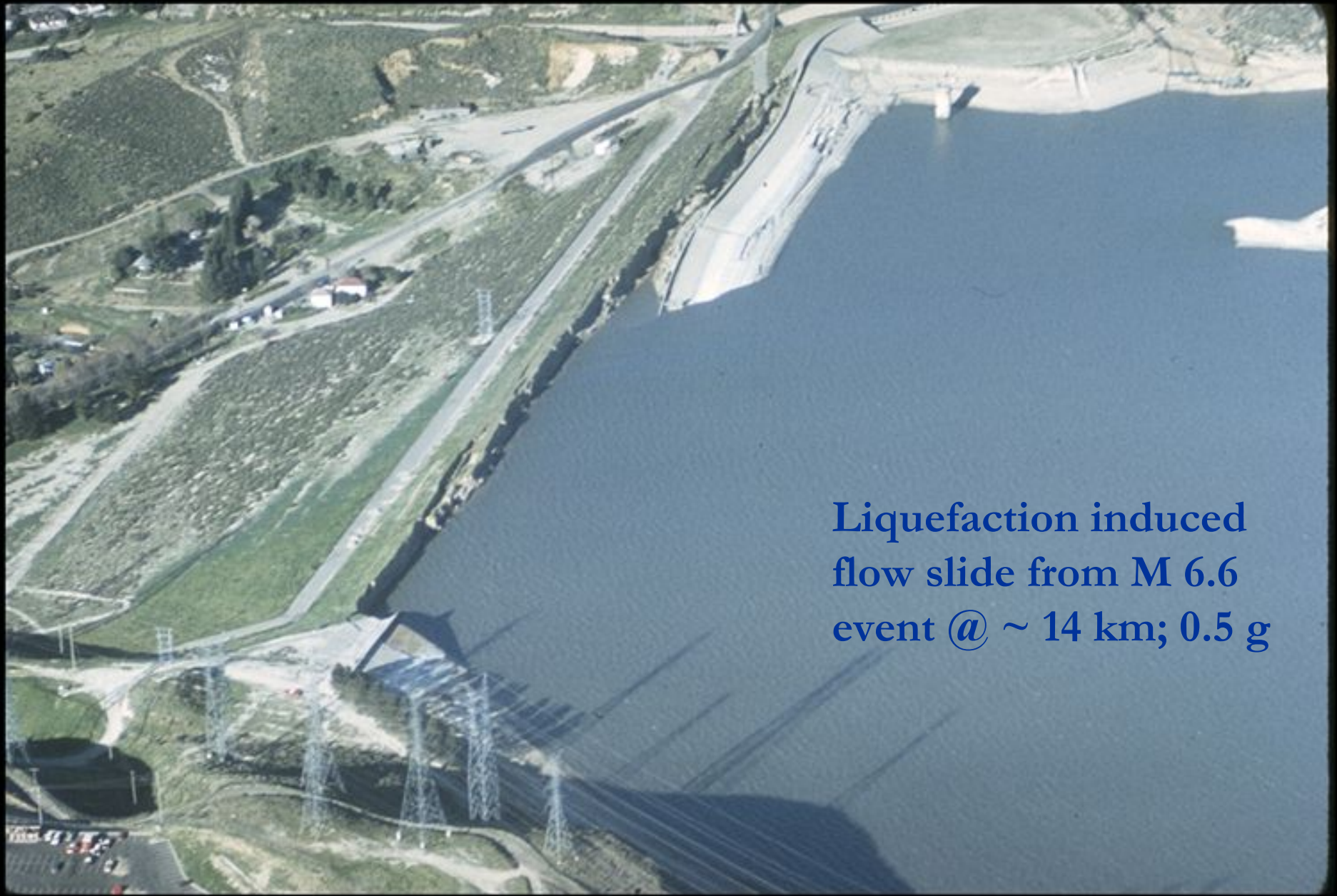


Probabilistic Seismic Hazard Assessment

Seismic Failure Modes

- Sliding failure through weak lift line
- Horizontal cracking
- Liquefaction of dam or foundation
- Cracking from severe shaking
- Surface fault displacement through the foundation
- Overtopping from landslide failure into reservoir





Liquefaction induced
flow slide from M 6.6
event @ ~ 14 km; 0.5 g





Fault surface rupture –
Chelongpu fault
M 7.6 Chi-Chi
earthquake, Taiwan



PSHA

“Probabilistic seismic hazard analysis (**PSHA**) is a methodology that estimates the likelihood that various levels of earthquake-caused ground motions will be exceeded at a given location in a given future time period. The results of such an analysis are expressed as estimated probabilities per year or estimated annual frequencies.”

PSHA

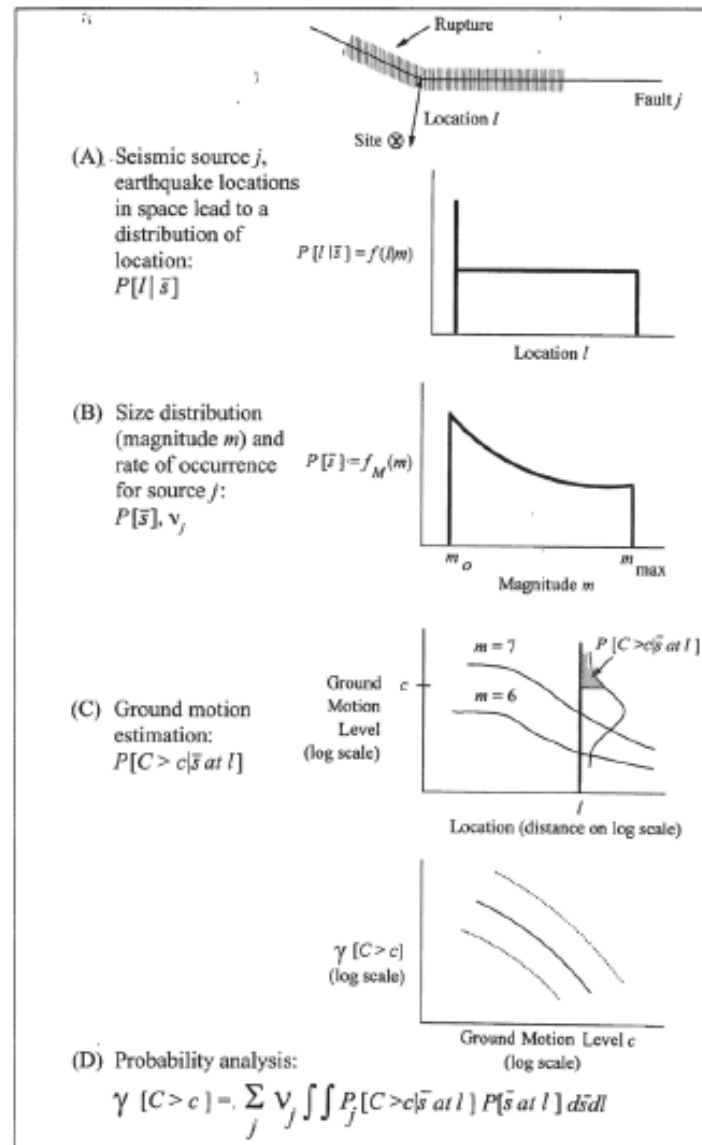
“While there is considerable information on earthquake ground motions and potential future locations of earthquakes, there is also considerable uncertainty in the inputs to the analysis.”

PSHA

“Recognizing the need to identify and address these uncertainties as part of a PSHA, the Senior Seismic Hazard Analysis Committee (SSHAC, 1997) established the goal for all PSHA’s to quantitatively assess these uncertainties and to represent the distribution of the informed technical community of alternative models and parameter values.”

PSHA

- Considers the contribution from all potential sources of earthquake shaking collectively
- Considers the likelihood of those events
- Uncertainty is treated explicitly
- Annual probability of exceeding specified ground motions is computed



Source: McGuire, 2004

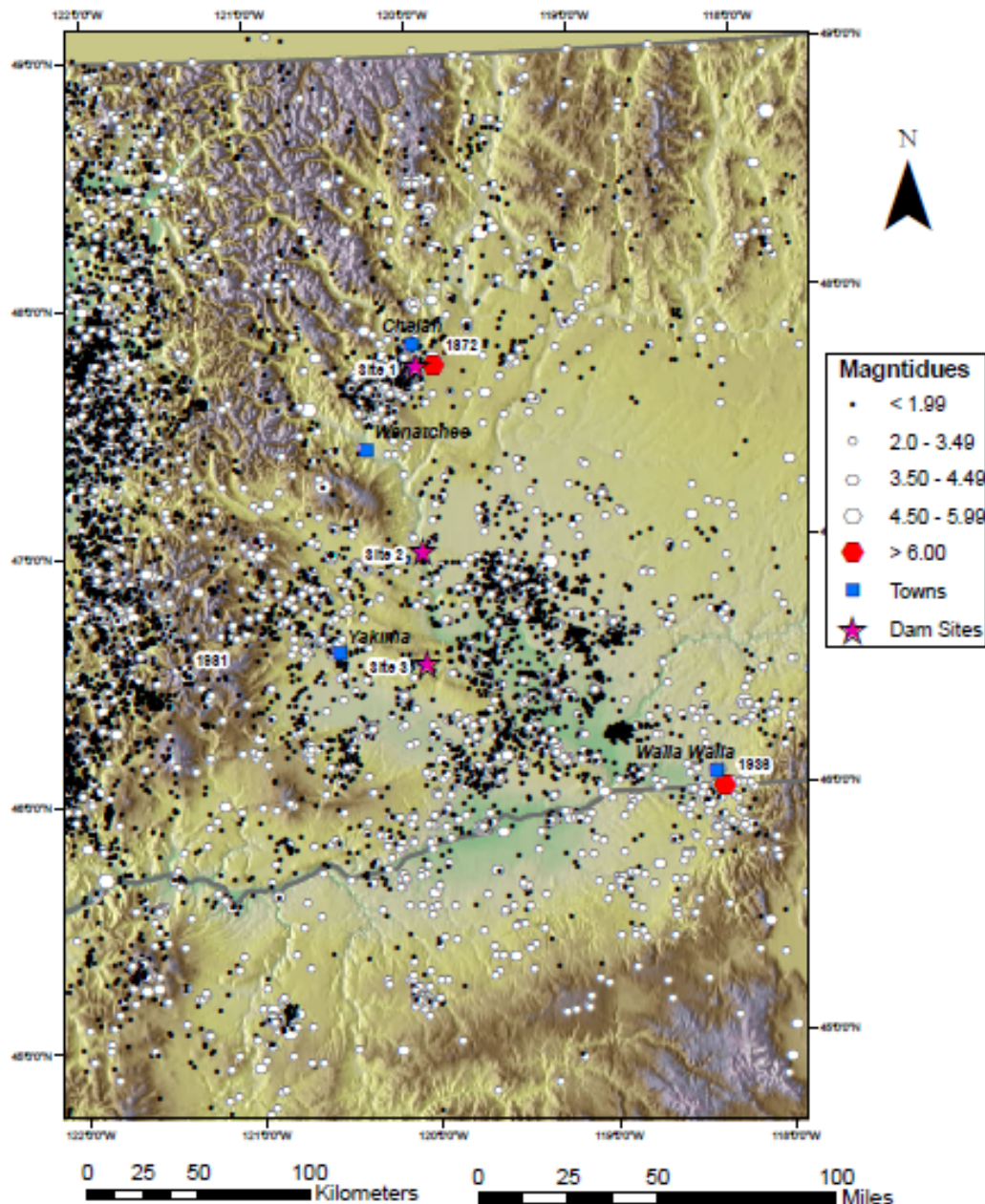
| | | |
|------------------------|--------------------------------|------------|
| Project No. 22238854 | THE STEPS IN PERFORMING A PSHA | Figure 2-1 |
| Mid-Columbia Dams PSHA | | |

Basic Components of a PSHA & All Seismic Hazard Analyses

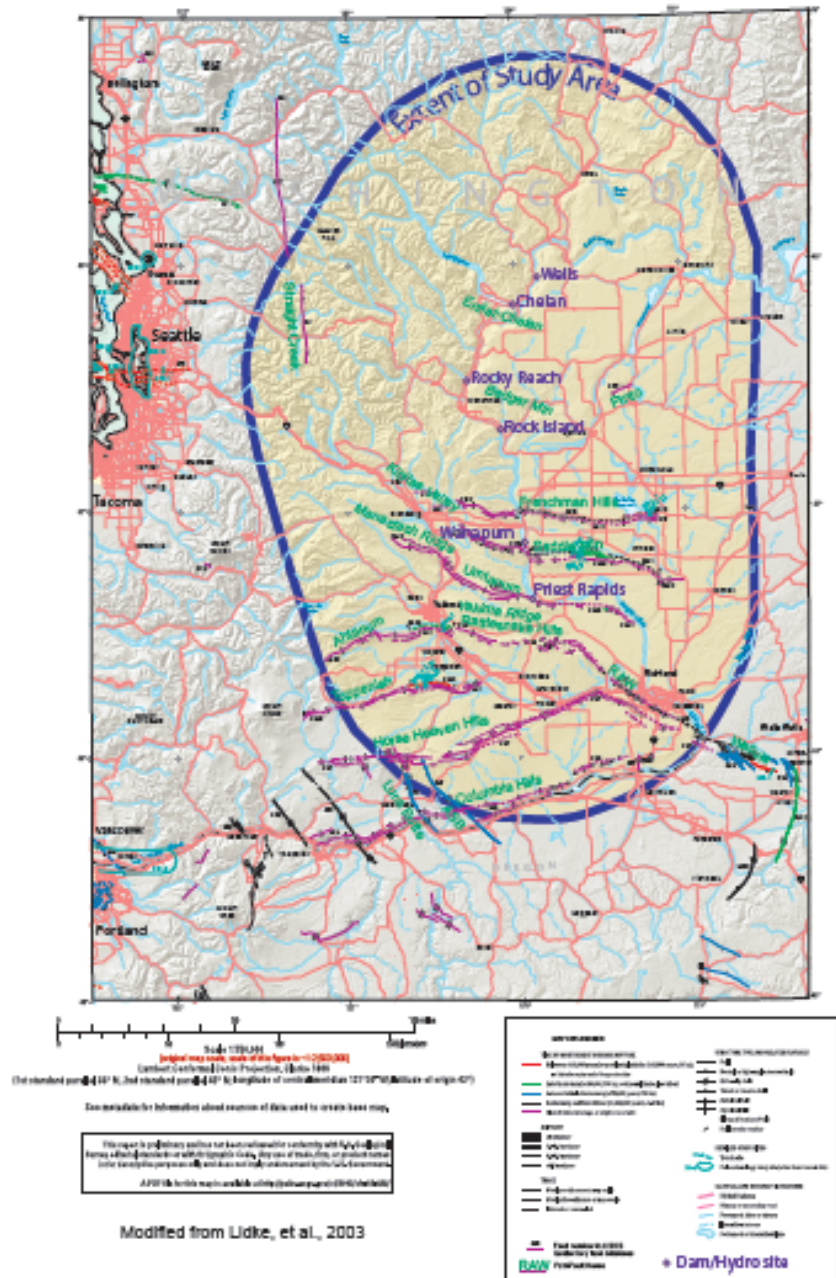
- I Seismic source characterization
- II Development of hazard curves
- III Development of uniform hazard spectra (UHS)
- IV Development of acceleration time histories

SEISMIC SOURCE CHARACTERIZATION

- Known faults
- Areal or Background sources (i.e., random seismicity)



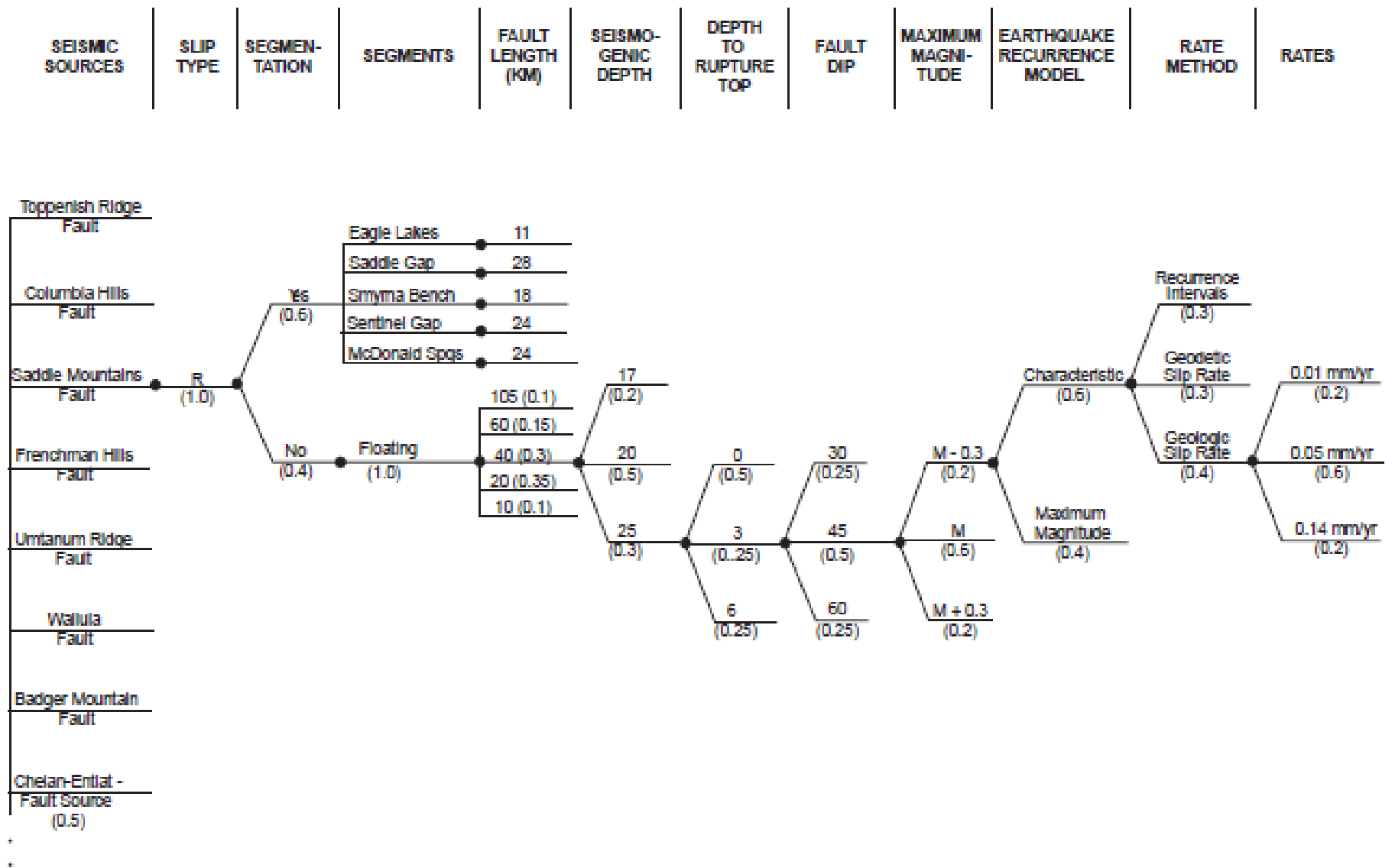
| | | | |
|--|---|--|---------------|
| | Project No. 22238854 Mid Columbia Dams PSHA | HISTORICAL SEISMICITY OF SOUTH-CENTRAL WASHINGTON AND NORTH-CENTRAL OREGON, 1841-2010 | Figure 1-2 |
|--|---|--|---------------|



Project No. 22238854
 Mid-Columbia Dams
 PSHA

PRELIMINARY MAP OF ACTIVE
 AND POTENTIALLY ACTIVE FAULTS,
 CENTRAL WASHINGTON

Figure
 1-1



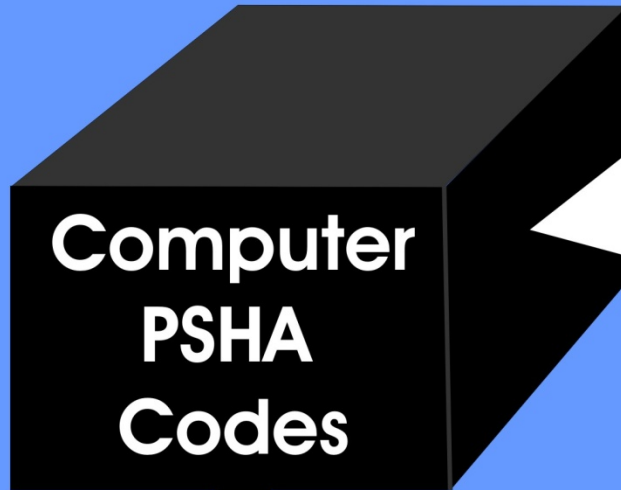
SEISMIC HAZARD CURVES

- Produced from several available computer codes
- Incorporate uncertainties in slip rate, magnitude, faults lengths
- Use ground motion attenuation relationships that relate PGA and SA to distance between source and site and earthquake magnitude - **NGA** Next Generation Attenuation models
- Site conditions very important; i.e., “soil” versus “rock”

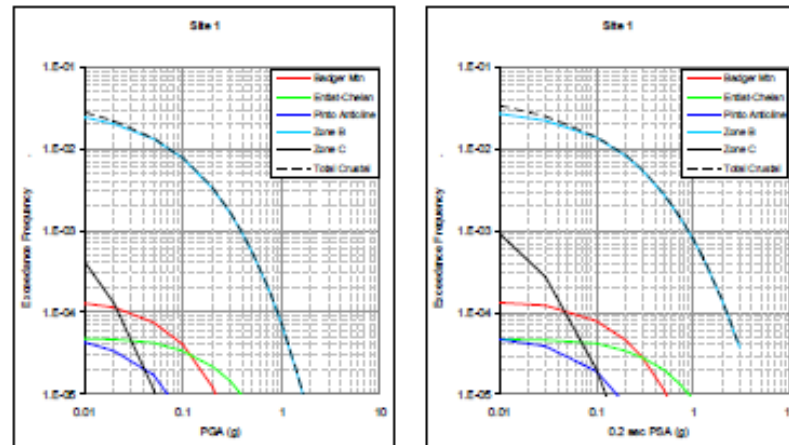
Earthquake Sources
(Faults & Seismicity)

**Ground Motion
Attenuation
Relations**

**Computer
PSHA
Codes**

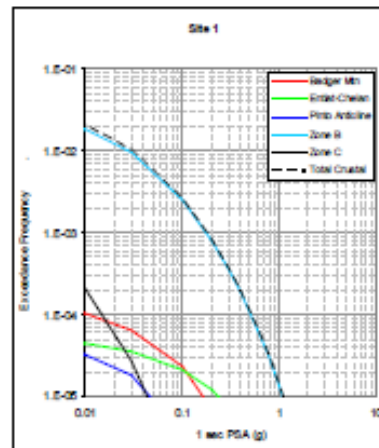


Mean Hazard Curves by Source



(a)

(b)



(c)

DRAFT

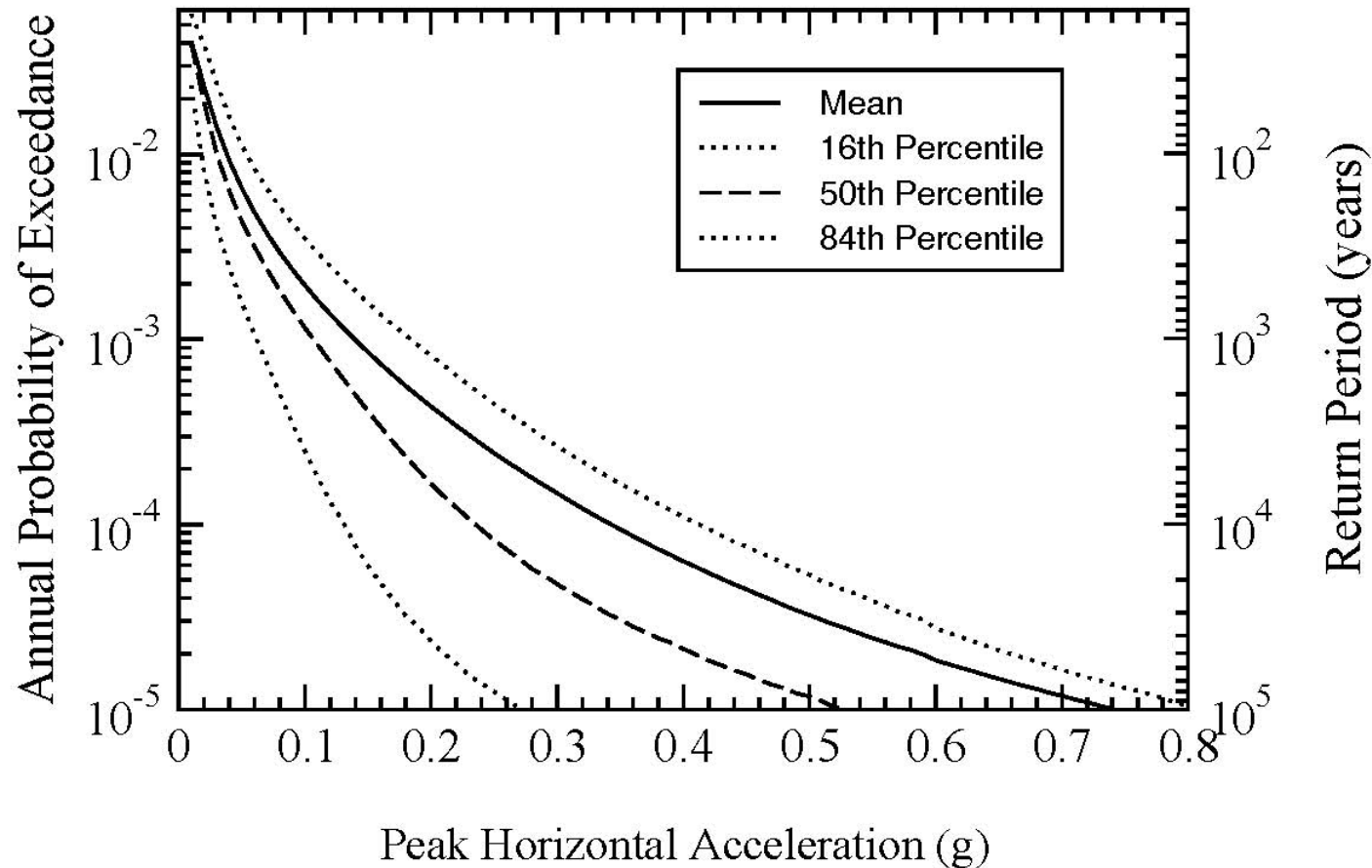
Project No. 22238854

Mid-Columbia Dam
PSHA

SEISMIC HAZARD RESULTS FOR SITE 1 FOR
INDIVIDUAL FAULTS AND SEISMIC SOURCE ZONES
FOR (a) PGA, (b) 0.2 SEC SA, AND (c) 1.0 SEC SA

Figure
6-4

Mean & Fractile Curves for PHA

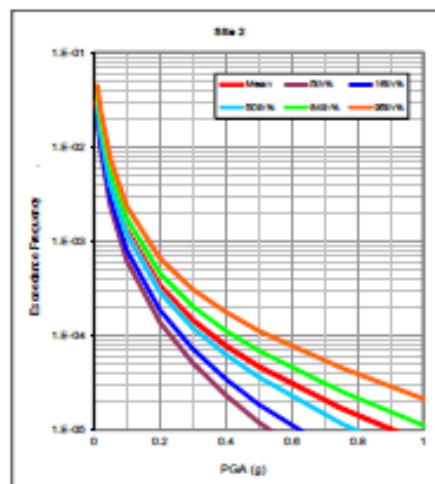


Site Response

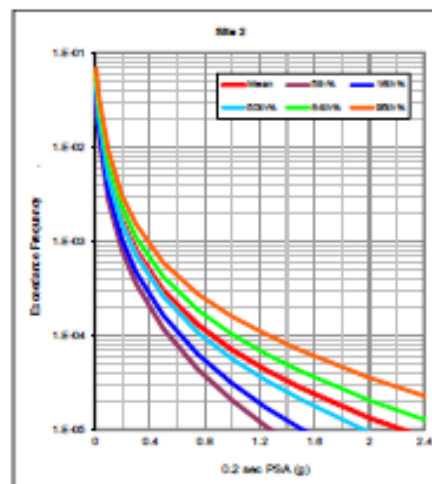
- Period of structure-
 - Concrete (sensitive to short period, 0.2-0.4-sec SA) verses Embankment (sensitive to long period, 1-sec SA)
- Site conditions-
 - Soil verses rock
- Shear wave velocity (V_{s30}) – Shear wave velocity in upper 100 ft - Important, because most NGA relationships now incorporate this)

UNIFORM HAZARD SPECTRA

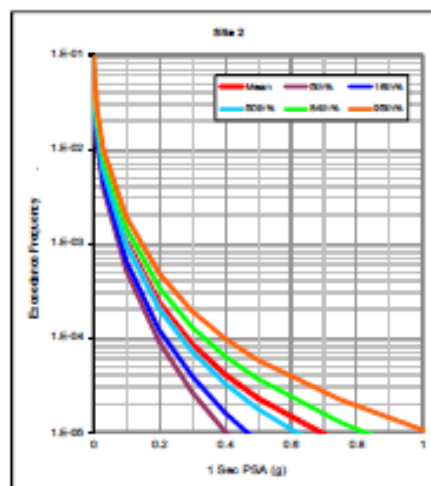
Uniform hazard spectra (UHS) are computed or developed from the seismic hazard curves. This is done by developing hazard curves (i.e. spectral acceleration vs. exceedance probability) for several vibration periods to define the response spectra. Then, for a given exceedance probability or return period, the ordinates are taken from the hazard curves for each spectral acceleration, and an “equal hazard” response spectrum is generated.



(a)



(b)



(c)

DRAFT

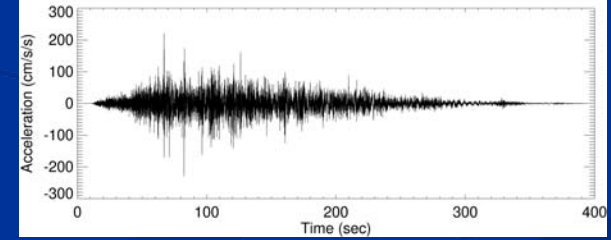
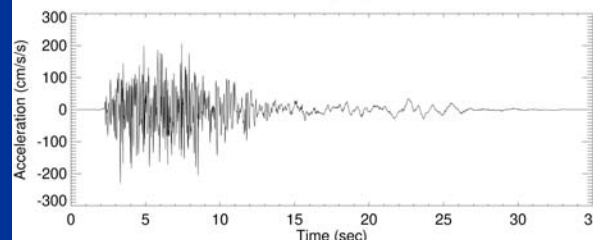
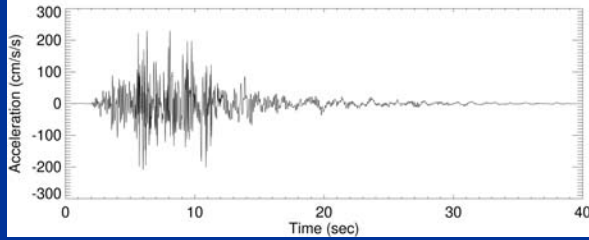
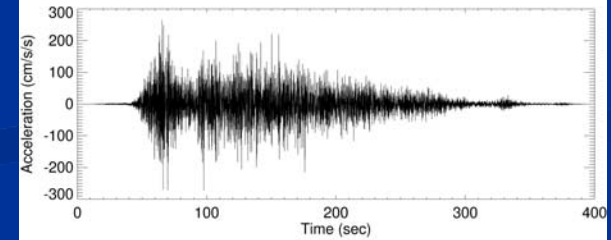
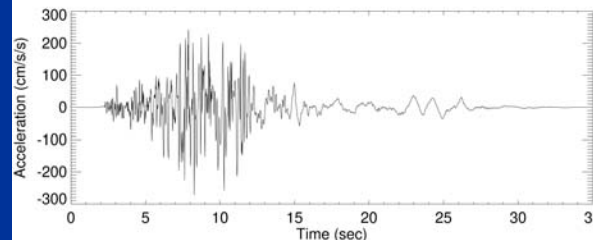
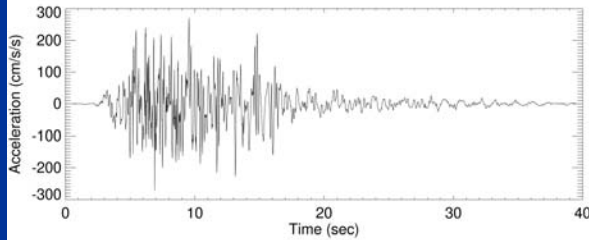
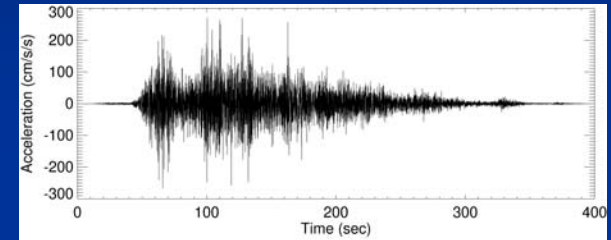
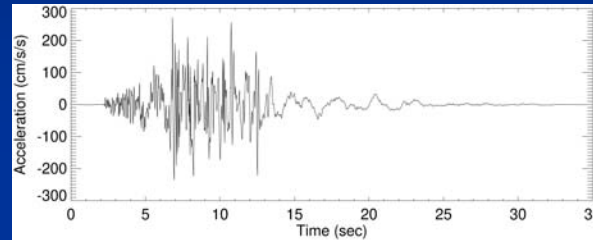
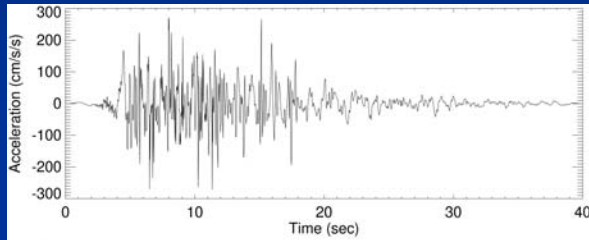
Project No. 22238854

Mid-Columbia Dams
PSHASEISMIC HAZARD RESULTS FOR SITE 2
FOR (a) PGA, (b) 0.2 SEC SA,
AND (c) 1.0 SEC SAFigure
6-12

TIME HISTORIES

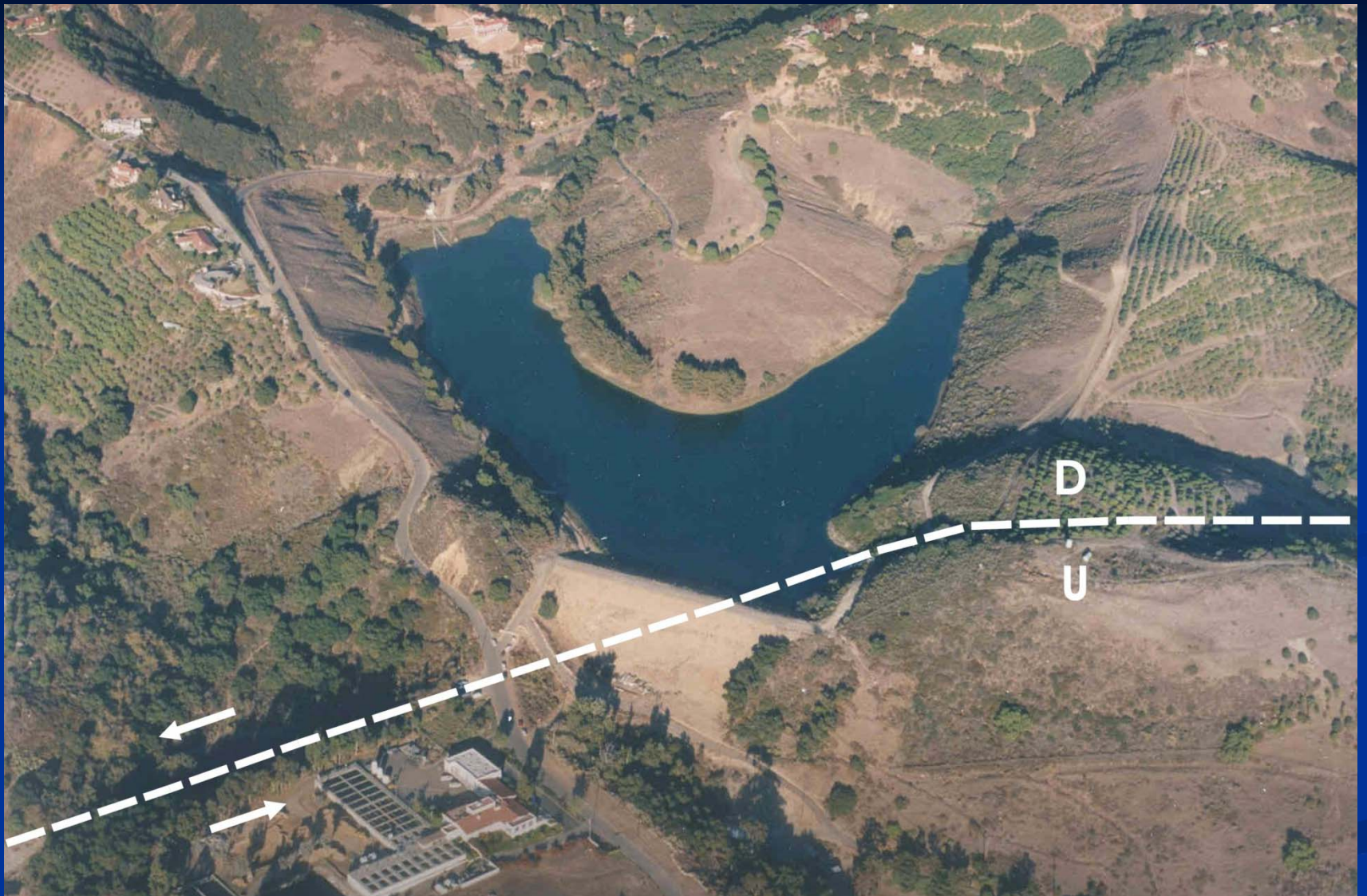
- For higher level studies, accelerograms, or acceleration time histories, are developed for the site that represents the seismic hazard at the return periods of interest.
- The suites of motions at each return period are usually selected to span the likely variability in spectra responses at different periods, and to account for differences in distance, magnitude, and site conditions. The selected ground motions are then used for dynamic analyses using programs such *FLAC*, *SHAKE*, or *LS-DYNA*

Time Histories (10K) Scaled to Mean UHS



Fault Displacement Hazard Analysis (PFDHA)

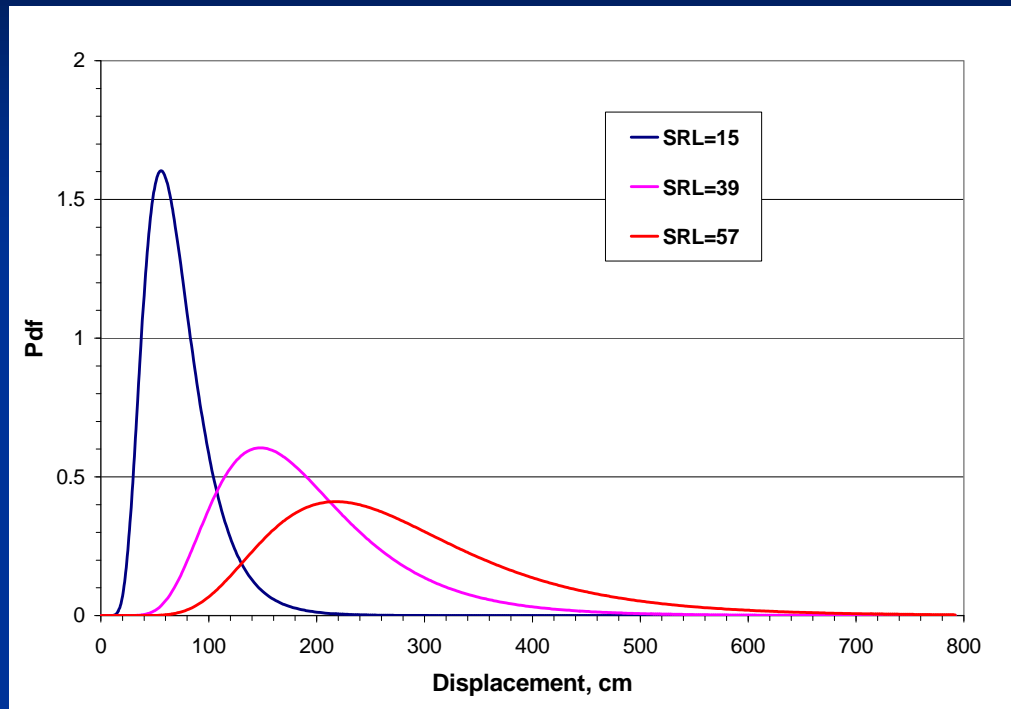
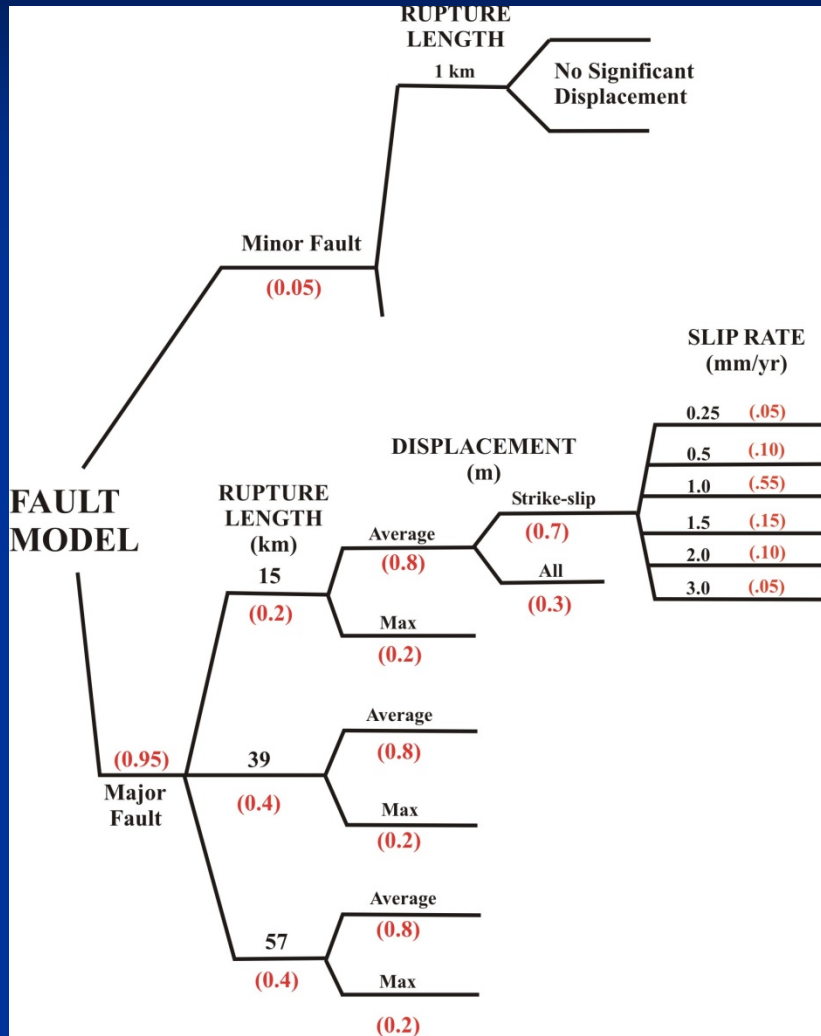
- Some sites it may be a major concern (i.e., Lauro & Terminal Dams, CA; Helena Valley Dam, MT)
- Hazards calculations are analogous to probabilistic ground motion methodology
- Methodology originally set forth by Stepp and others (2001) for Yucca Mtn



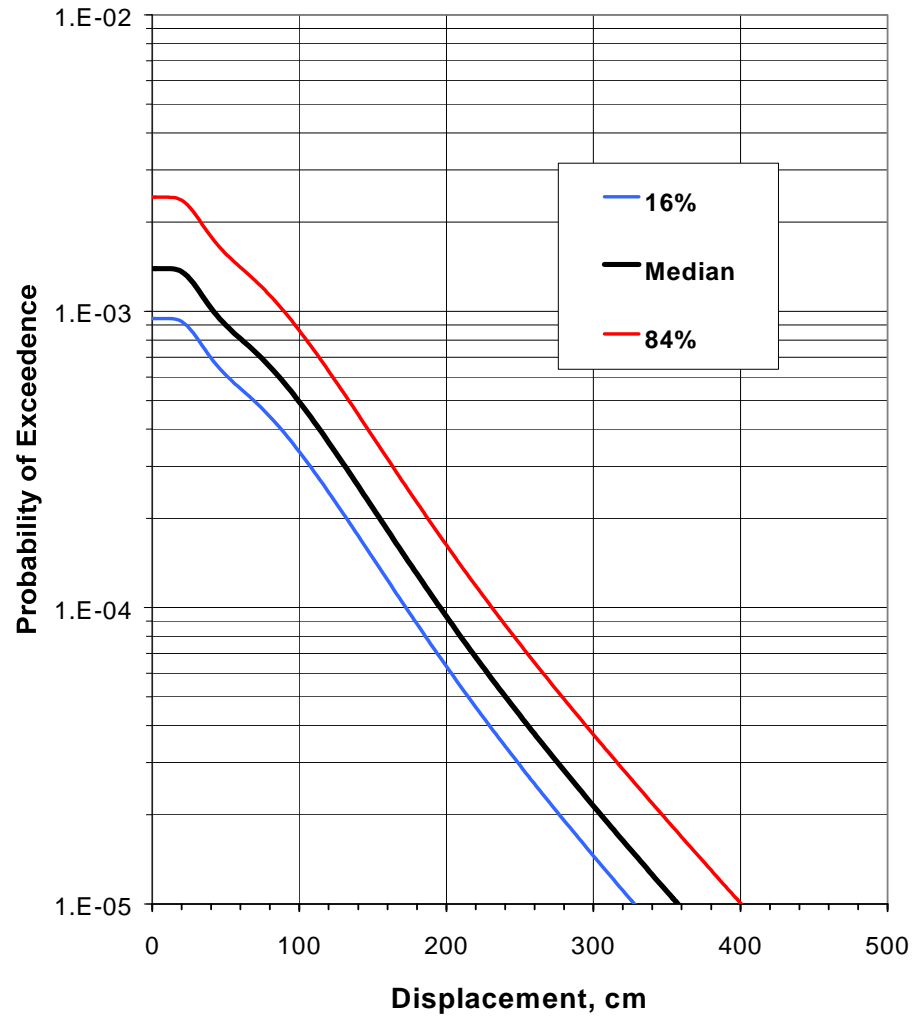
Lauro Dam Fault Investigations

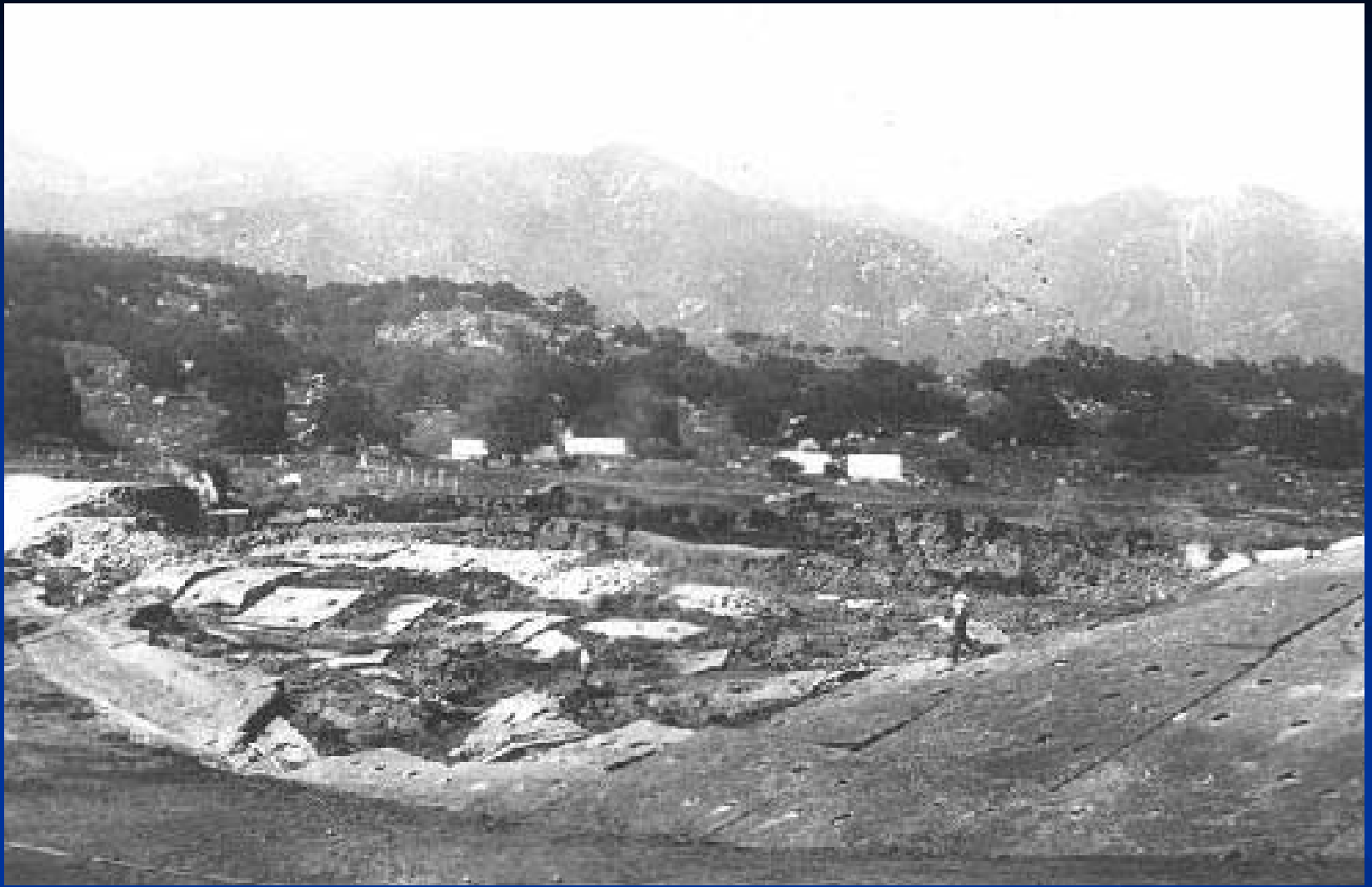


Rupture Length vs Displacement



Exceedence (All)





QUESTIONS?

Sheffield Dam - 1925